
An Effective and Stabilization of Expansive Soils using Flyash

Syed Saba Sulthana

M.Tech, Department of Civil Engineering

ABSTRACT: *Virtually 51.8 million hectares of land discipline in India are blanketed with Expansive soil (in most cases Black Cotton soil). The property of those expansive soils, more often than not, is that they're very tough when within the dry state, however, they lose all of their strength when in the wet state. In gentle of this property of expansive soils, these soils pose problems international that serve as a project to beat for the Geotechnical engineers. Broad laboratory/area trials were carried out via quite a lot of researchers and have proven promising outcome for software of such expansive soil after stabilization with additives comparable to sand, silt, lime, fly ash, etc. As fly ash is freely on hand, fortasks within the vicinity of Thermal power crops, it can be used for stabilization of expansive soils for various makes use of. The gift paper describes a be taught implemented to examine the upgrades in the houses of expansive soil with fly ash in various percentages. Both laboratory trials and field test had been implemented and the outcome is suggested in this paper. Some of the fundamental difficulties in subject software are thorough mixing of the two substances (expansive soil and fly ash) in required proportion to form a homogeneous mass. The paper describes a method adopted for putting these materials in layers of required thickness and working a "Disc Harrow.*

KEYWORDS- Compaction, field tests, fly ash, laboratory tests, plastic clay, stabilization

I. INTRODUCTION

Expansive soils, which are also called as swell-shrink soil, have the tendency to shrink and swell with variation in moisture content. As a result of this variation in the soil, significant distress occurs in the soil, which is subsequently followed by damage to the overlying structures. During periods of greater moisture, like monsoons, these soils imbibe the water, and swell; subsequently, they become soft and their water holding capacity diminishes. As opposed to this, in drier seasons, like Summers, these soils lose the moisture held in them

due to evaporation, resulting in their becoming harder. Generally found in semi-arid and arid regions of the globe, these types of soils are regarded as a potential natural hazard – if not treated, these can cause extensive damage to the structures built upon them, as well causing loss of human life. Soils whose composition include the presence of montmorillonite, in general, display this kind of properties. Tallied in billions of dollars annually worldwide, these soils have caused extensive damage to civil engineering structures. Also called as Black Cotton soils or Regur soils, expansive soils in the Indian subcontinent are mainly found over the Deccan trap (Deccan lava tract), which includes Maharashtra, Andhra Pradesh, Gujarat, Madhya Pradesh, and some scattered places in Odisha. These soils are also found in the river valley of the Narmada, Tapi, Godavari and Krishna. The depth of black cotton soil is very large in the upper parts of the Godavari and Krishna, and the northwestern part of Deccan Plateau. Basically, after the chemical decomposition of rocks such as basalt by various decomposing agents, these are the residual soils left behind at the place of such an event. Cooling of volcanic eruption (lava) and weathering another kind of rock – igneous rocks – are also processes of formation of these types of soils. Rich in lime, alumina, magnesia, and iron, these soils, lack of nitrogen, phosphorus and organic content.

Consisting of high percentage of clay sized particles, the color of this soil varies from black to chestnut brown. 20% of the total land area, on an average, of this country is roofed by expansive soils. These soils are suitable for dry farming and for the growth of crops like cotton, rice, jowar, wheat, cereal, tobacco, sugarcane, oilseeds, citrus fruits and vegetables; the reason behind it is owed to the moisture retentive capacity of expansive soils, which is high. In the semi-arid regions, just in the last couple of decades, damages due to the swelling and shrinking action of expansive soils have been observed prominently in the form of cracking and break-up of roadways, channel and reservoir linings, pavements, building foundations,

waterlines, irrigation systems, sewer lines, and slab-on-grade members.

II. RELATED WORKS

A waste material extracted from the gases emanating from coal-fired furnaces, generally of a thermal power plant, is called fly ash. One of the chief usages of volcanic ashes in the ancient ages was the use of it as hydraulic cement, and fly ash bears the closest resemblance to these volcanic ashes. These ashes were believed to be one of the best pozzolans (binding agent) used in and around the globe.

The demand for power supply has exponentially heightened these days due to increasing urbanization and industrialization phenomena. Subsequently, this growth has resulted in the increase in the number of power supplying thermal power plants that use coal as a burning fuel to produce electricity. The mineral residue that is left behind after the burning of coal is the fly ash. The Electro Static Precipitator (ESP) of the power plants collect these fly ashes. Production of fly ash comes with two major concerns – safe disposal and management of fly ash. Because of the possession of the complex characteristics of wastes which are generated from the industries, and their hazardous nature, these wastes pose a necessity of being disposed of in a safe and effective way, so as to not disturb the ecological system, and not causing any sort of catastrophe to human life and nature. Environmental pollution is imminent unless these industrial wastes are pre-treated before their disposal or storage.

Essentially consisting of alumina, silica, and iron, fly ashes are micro-sized particles. Fly ash particles are generally spherical in size, and this property makes it easy for them to blend and flow, to make a suitable concoction. Both amorphous and crystalline nature of minerals is the content of fly ash generated. Its content varies with the change in the nature of the coal used for the burning process, but it basically is a non-plastic silt. For waste liners, fly ash is a potential material that can be employed; and in combination with certain minerals (lime and bentonite), fly ash can be used as a barrier material. In the present scenario, the generation of this waste material in the picture (fly ash) is far more than its current utilization. In other words, we are producing more of fly ash than we can spend.

III. THE PROPOSED APPROACH

Following laboratory tests have been carried out as per IS: 2720. The tests were carried out both on natural soil and stabilized soil with fly ash collected from Ennore Thermal Power Plant.

- (i) Grain Size Analysis
- (ii) Atterberg Limit Test
- (iii) Proctor Compaction Test
- (iv) Unconfined Compression Test
- (v) Permeability Test

After removing impurities like vegetation, stones, etc. the soil was mixed with fly ash in varying proportion by volume. The mixing was thoroughly carried out manually and the tests were conducted as per standard procedures. The liquid limit and plastic limit of the soil with varying percentage of fly ash is given in Table 1. The proctor tests carried out is summarized in Fig. 1.

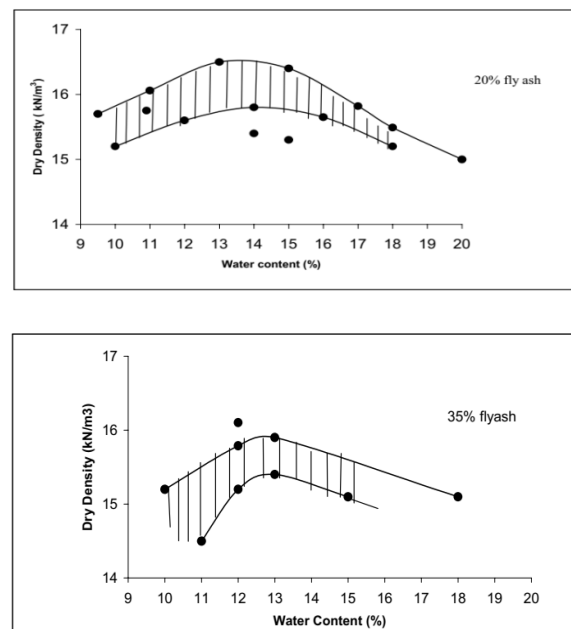


Fig 1. Typical relationship between dry density and water content

The grain size analysis of the borrow soil and the fly ash is shown in Fig. 2. Unconfined compression strength tests have been carried out on cylindrical samples of 36 mm diameter and 72 mm high prepared using miniature compaction apparatus with 15% moisture content. The samples were allowed to cure by air drying for 15 days. The samples were tested with a constant strain rate of 0.625 mm/min.

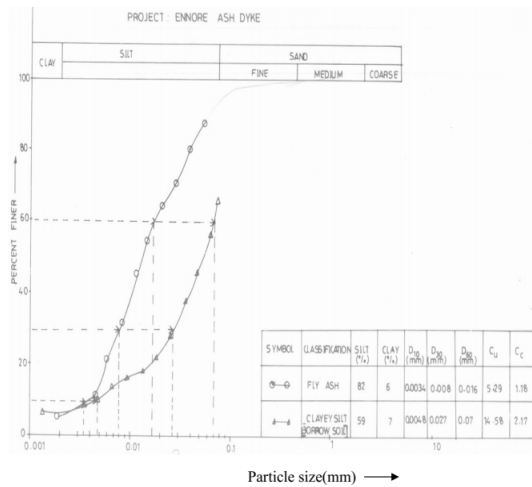


Fig 2. Grain size analysis of borrow soil and fly ash

IV. EXPERIMENTAL SETUP

Field trails were carried out by constructing an embankment measuring 3 to 4m wide and 30m long. The height of the embankment was about 600mm. Each layer of 200mm loose thickness was placed with the varying fly ash content. To achieve the desired fly ash content, the layers were placed such that the fly ash layer is sandwiched between two soil layers as per the details given in Table 4. For each trial mix, the required thickness of borrow soil was manually spread first. Above this, fly ash collected from the ESP of the Thermal Power Plant was spread. This was again followed by a third layer of soil. The layer of fly ash was sandwiched between two layers of soil to prevent it from flying off.

After this, a disc harrow equipment shown in Fig. 3 was used for uniform mixing of soil and fly ash. This equipment is a circular disc, which penetrates through the loosely placed layers and pulled horizontally by a tractor. The discs rotate in such a fashion that the soil is shuffled and mixed thoroughly. It was observed at a site that after about eight passes of the disc harrow, the dry mixing of the two materials was quite satisfactory and with uniform colour of the mix. After this, the required quantity of water was manually sprayed over the layer to achieve the required moisture content of 15%, 6 passes of disc harrow were made for uniform mixing of additional water with the material already mixed. After 6 passes, the mixing of moisture was found to be uniform. Though a sheep foot roller is ideally suited for compaction of plastic

clay, after mixing with fly ash, there was considerable improvement in the workability, the compaction was therefore carried out with a 12 tonne smooth wheel roller. Each layer of mix, prepared as above was compacted with 8 passes of the roller. The material after compaction was found to be quite hard and no significant penetration of the roller wheel was noticed during the last 2 passes. After compaction the thickness of the layer (initial loose thickness of 200mm) was found to be 120 to 130mm. Fig. 4 shows a view of a compacted layer. To check the adequacy of compaction, following control tests were carried out on each of the compacted layers.

- (i) In-site density by core cutter
- (ii) Natural moisture content
- (iii) Light cone penetration test

TABLE 1: Atterberg limits of Soil–Flyash mixtures

Soil Type	Liquid Limit (%)	Plastic Limit (%)	Plasticity index (%)
Soil alone	30	18	12
Soil + 10% FA	28	20	8
Soil + 20% FA	29	19	10
Soil + 25% FA	30	19	11
Soil + 30% FA	30	21	9
Soil + 40% FA	NA	NA	NA



Fig 3. Soil flyash mixing with a disc harrow



Fig.4 View of compacted surface (soil with 20% FA)

V. CONCLUSION

The ordinary soil used for construction shall be dried with the moisture content material below 7%. If the soil has extra moisture it's problematic to mix with FA. Such soil can be spread on the surface and allowed to dry before construction. Presence of dry clay lumps in the borrow soil raises the quantity of passes of disc harrow for mixing. It is, thus, crucial to do away with such soil lumps in the development. It is found that inserting of two unique materials (local soil and FA) in three layers with FA layer sandwiched between soil layers and mixing them with disc harrow is workable. It's most popular to cover the compacted soil-FA bound with a compatible soil cover of minimal 500mm thickness. For this rationale suitable borrow soil of CI variety (in limited variety) will probably be used.

REFERENCES

- [1] Chen, F. H. (1988), "Foundations on expansive soils", Chen & Associates, Elsevier Publications, U.S.A.
- [2] Erdal Cokca (2001) "Use Of Class C Fly Ashes for the Stabilization – of an Expansive Soil" Journal of Geotechnical and Geoenvironmental Engineering Vol. 127, July, pp. 568-573.
- [3] Pandian, N.S., Krishna, K.C. & Leelavathamma B., (2002), Effect of Fly Ash on the CBR Behaviour of Soils, Indian Geotechnical Conference, Allahabad, Vol.1, pp.183-186.
- [4] Phanikumar B.R., & Radhey S.Sharma (2004) "Effect of flyash on Engg properties of Expansive Soil" Journal of Geotechnical and Geoenvironmental Engineering Vol. 130, no 7, July, pp. 764-767.
- [5] Prashanth J.P., (1998) "Evaluation of the Properties of Fly Ash for its Use in Geotechnical Applications". Ph.D Thesis, IISC. Bangalore.
- [6] Raymond N.Yong and Benno P.Warkentin (1975) "Soil Properties and Behaviour" Elsevier Publications, U.S.A.
- [7] Radhakrishnan, G., Kumar, M.A., and Raju, G.V.R.P. (2014), "Swelling Properties of Expansive Soils Treated with Chemicals and Fly ash", American Journal of Engineering Research. Vol. 3, Issue 4, pp. 245-250.
- [8] Raut, J.M., Bajad, S.P., and Khadeshwar, S.P. (2014), "Stabilization of Expansive Soils Using Fly ash and Murrum", International Journal of Innovative Research in Science, Engineering and Technology. Vol. 3, Issue 7.
- [9] Satyanarayana P.V.V., Kumar, S.H., Praveen, P., Kumar, B.V.S. (2013), "A Study on Strength Characteristics of Expansive Soil-Fly ash Mixes at Various Moulding, Water Contents", International Journal of Recent Technology and Engineering. Vol. 2, Issue 5.
- [10] Senol, A., Etminan. E., and Olgun, C. (2012), "Stabilization of Clayey Soils Using Fly Ash and Homopolymer polypropylene", GeoCongress, pp. 3929-3938.

Bio-data:



Syed Saba Sulthana completed M.Tech(Geotechnical Engineering), Department of Civil Engineering.