

Seismic retrofit of reinforced concrete buildings using jacketing

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Abstract:

Many parts of the country have suffered earthquake in last three decades. In coastal part of South India faced Tsunami. In first three earthquakes it was found that many of damaged structures were built in non-engineered masonry techniques. Unreinforced masonry structures are the most vulnerable during an earthquake. Normally they are designed for vertical loads and since masonry has adequate compressive strength, the structures behave well as long as the loads are vertical. When such a masonry structure is subjected to lateral inertial loads during an earthquake, the walls develop shear and flexural stresses. In previous earthquakes many R.C.C buildings have also collapsed and are found unsafe due to faulty workmanship. Many other causes are responsible for major collapse and damage to the R.C.C structures. This thesis presents a study of PL+SC+S+G+5 storey building, where this PL+SC+S+G+5 storey residential building is being converted to commercial building which results increase of live load in existing building. This overall analysis is done by using ETABS software. We follow the technique of retrofitting by jacketing method to increase resistance of the building towards lateral forces.

Keywords: PL+SC+S+G+5 story, ETABS, dead loads, live loads, retrofitting, jacketing etc.

1. Introduction

When the normal residential building is being changed to general commercial building that gives results in increasing the live load in existing building, this was proven in physical and experimental investigations also. It was concluded that the buildings either should

be demolished or at least should be redefined with suitable techniques, to increase its service life. This can be overcome by new technique called retrofitting by jacketing. It was decided to implement reinforced cement concrete column jacketing technique due to its feasibility and ease for execution of the existing building. All the columns and beams on all the floors are now suitably jacketed. The loose pockets of concrete that were investigated during the test are re-concreted, and rusted reinforcement is replaced with new reinforcement as per the given design requirements.

Redefining utilizes the concepts of confinements by strengthening the existing beam or column with external pressures. Confinement in reinforced concrete also uses the lateral steel reinforcement to delay the lateral expansion of the confined concrete. This is known as the concrete core. While strengthening the longitudinal steel and reinforcement by brace increasing and its buckling strength. Certain reinforcement concrete columns are also designed and utilizing the concept of confinement with steel rebar hoops or spirals inside the concrete core. The need for additional confinement is sometimes necessary. It is also important to know how new retrofit material will enhance and strengthen the existing member.

Objectives

So many researchers have been developed equations and models to predict the behavior of the reinforced concrete columns confined with steel rebar, the

continuous steel bar, concrete jacketing, or even fibre composite material. The purpose of this thesis is to examine the existing normal residential building and to retrofit to general commercial building as increases in live load. We can do the retrofitting in various ways as in here adopted the retrofitting by jacketing method. There is a scope of retrofitting for existing building with various methods depending upon various parameters like functionality, cost and type of structure. Retrofitting is technical evaluation in structural system of the building that improves the resistance to earthquake by optimizing the strength of the building, ductility and earthquake loads. Strength of the building is generated from the structural dimensions, materials used, shape of the structure, and number of structural elements like beams and columns etc. Ductility of the building is generated from good detailing, materials used, degree of seismic resistant, etc. Earthquake load is generated from the site seismicity, mass of the structures, important of buildings, degree of seismic resistant, etc. Due to the variety of structural condition of building, it is hard to develop typical rules for retrofitting. Each building has different approaches depending on the structural deficiencies. So there is a need to prepare and design the retrofitting approaches. In the design of retrofitting approach, the engineer must comply with the building codes. The results generated by the adopted retrofitting techniques must fulfill the minimum requirements on the buildings codes, such as deformation, detailing, strength, etc.

2. Literature review

E. Senthil Kumar, A. Murugesan And G.S. Thirugnanam et al., Did an experimental investigation of the behavior of retrofitted FRP (fibre reinforced polymer) wrapped exterior beam-column joint of a G+4 building in Salem, which lies in seismic zone III. The test specimen was taken to be one fifth model of beam column joint from the prototype specimen and was evaluated in terms of load displacement relation, ductility, stiffness, load ratio and cracking pattern. On comparing the test results with the analytical modeling of the joint on ANSYS and STAAD Pro, it was found that such external confinement of concrete increased the load carrying capacity of the control specimen by 60% and energy absorption capacity by 30-60%.

Durgesh C. Rai: Gave the guidelines for seismic evaluation and strengthening of buildings. This document is developed as part of project entitled Review of Building Codes and Preparation of

Commentary and Handbooks awarded to Indian Institute of Technology Kanpur by the Gujarat State Disaster Management Authority (GSDMA), Gandhi Nagar through World Bank finances. This document is particularly concerned with the seismic evaluation and strengthening of existing buildings and it is intended to be used as a guide.

Yen-Po Wang introduced the fundamentals of seismic base isolation as an effective technique for seismic design of structures. Spring-like isolation bearings reduce earthquake forces by changing the fundamental time period of the structure to avoid resonance. However, sliding-type isolation bearings filter out the earthquake forces via discontinuous sliding interfaces and forces are prevented from getting transmitted to the superstructure because of the friction. The design of the base isolation system includes finding out the base shear, bearing displacement etc. in accordance with site-specific conditions.

3. Methods of Retrofitting

Increase the capacity/strength of the system (Seismic Resistance Based Design)

- (i) Concrete Jacketing.
- (ii) Steel Jacketing.
- (iii) FRP Wrapping



Fig 1. Column jacketing at basement stage



Fig 2. Beam column junction strengthening in basement

4. Software review of ETABS

ETABS is a sophisticated, but easy to use, for analysis and design purpose. ETABS program is

developed specifically for building systems (structural). ETABS features are intuitive and have powerful graphical interface coupled with unmatched modeling, analytical, and design procedures, all integrated using a common database. ETABS can also handle most complex building structural models which includes a wide range of nonlinear behaviors, making it as tool of choice for structural engineering by structural engineer is perfect in the building industry.

5. modeling with Etabs

Here a G+5 storey building that is Existing Building before Retrofitting modeled by using ETABS 9 software. The detailed features of the normal building are given below.

Features of building (model 1)

Structure = Before retrofitting of the building (structure 1)

Floors = Plinth level + Sub cellar + Cellar +Ground + 5

Grid lines in X-direction = 1

Grid lines in Y-direction = 12

Grid width in X-direction = 19.75 m

Grid width in Y-direction = The grid width of building in y-direction varies as per the plan

Live load on slab = 2.0 kN/m²(all floors except terrace floor)

= 1.5 kN/m² (terrace floor)

Dead load on slab = 2.0 kN/m²(all floors except terrace floor)

= 1.5 kN/m² (terrace floor)

Floor Finish = 1.0 kN/m²

Water proofing = 1.0 kN/m²

Storey height = 3.2 m

Wall Thickness = 0.25 m (exterior wall)

= 0.15 m (interior wall)

Thickness of Slab = 0.15 m

Grade of concrete = M25 (For beams)

= M30 (For columns)

Grade of steel = Fe415

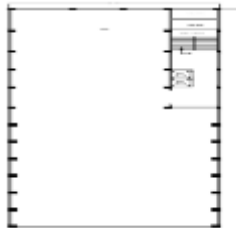


Fig 3: plan view



Fig 4 3d view

$$\begin{aligned} \text{Base Shear of Building} &= Ah * W \\ &= 0.0666 * 104125.082 \\ &= 6934.730 \text{ kN} \end{aligned}$$

Model 2

Structure = Existing Building after Retrofitting (Structure 2)

Floors = Plinth level + Sub cellar + Cellar +Ground + 5

Grid in X-direction = 1

Grid in Y-direction = 12

Grid width in X-direction = 19.75m

Grid width in Y-direction = The grid width of building in y-direction varies as per the plan

Live load on slab = 6.0 kN/m²(all floors except terrace floor)

= 2.5 kN/m² (terrace floor)

Dead load on slab = 2.0 kN/m²(all floors except terrace floor)

= 1.5 kN/m² (terrace floor)

Floor Finish = 1.0 kN/m²

Water proofing = 1.0 kN/m²

Storey height = 3 m

Wall Thickness = 0.25 m (exterior wall)

= 0.15 m (interior wall)

Thickness of Slab = 0.15 m

Grade of concrete = M25 (For beams)

= M30 (For columns)

Grade of steel = Fe415



Fig 5: 3D view

$$\begin{aligned} \text{Base Shear of Building} &= Ah * W \\ &= 0.0666 * 101449.098 \\ &= 6756.5099 \text{ KN} \end{aligned}$$

6. Retrofitting by jacking method Jacketing of Columns

Jacketing of columns consists of added concrete with longitudinal and transverse reinforcement around the existing columns. This type of strengthening improves the axial and shear strength of columns while the flexural strength of column and strength of the beam-column joints remain the same. It is also observed that the jacketing of columns is not successful for improving the ductility. A major advantage of column jacketing is that it improves the lateral load capacity of the building in a reasonably uniform and distributed way and hence avoiding the concentration of stiffness as in the case of shear walls. This shows major strengthening of foundations may be avoided. In addition the original function of the building can be maintained, as there are no major changes in the original geometry of the building with this technique. The jacketing of columns is generally carried out by two methods:

- (i) reinforced concrete jacketing
- (ii) Steel jacketing

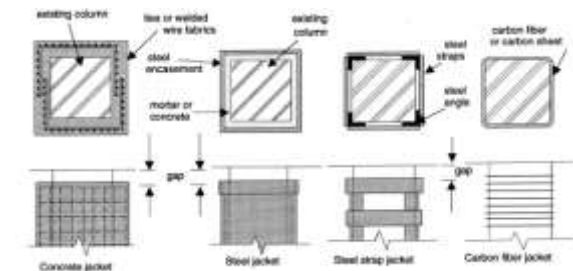


Fig 6 :Types of jacketing in seismic retrofitting

Reinforced Concrete Jacketing:

Reinforced concrete jacketing can be employed as a repair or strengthening scheme. Damaged regions of the existing members should be repaired prior to their jacketing. There are two main purposes of jacketing of columns:

- (i) Increase in the shear capacity of columns in order to accomplish a strong column weak beam design and
 - (ii) To improve the column's flexural strength by the longitudinal steel of the jacket made continuous through the slab system are anchored with the foundation. It is achieved by passing the new longitudinal reinforcement through holes drilled in the slab and by placing new concrete in the beam column joints.
- Rehabilitated sections are designed in this way so that the flexural strength of columns should be greater

than that of the beams. Transverse steel above and below the joint has been provided.

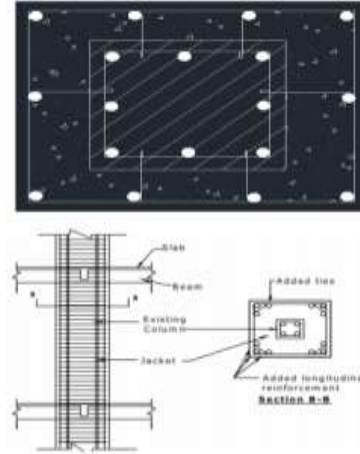
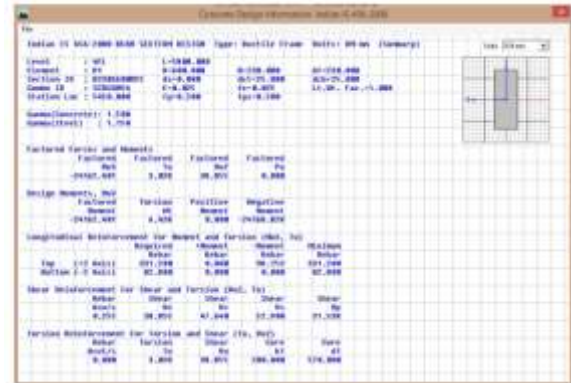
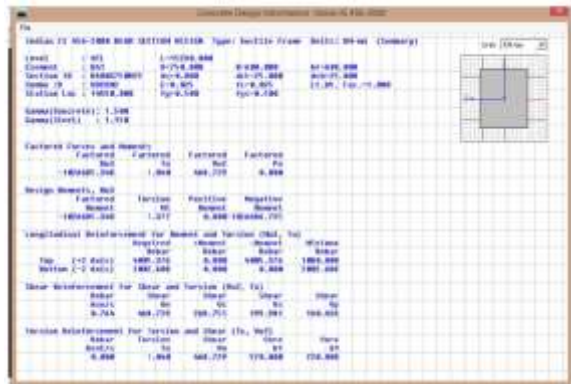


Fig. 7: Construction Technique for Column Jacketing

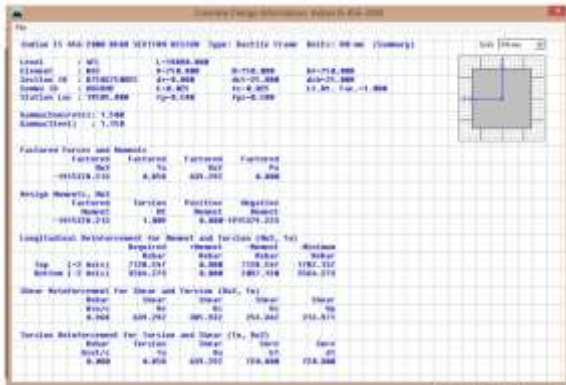
Design detail of beam for existing building



Detailing of beam for existing building (B230X600)

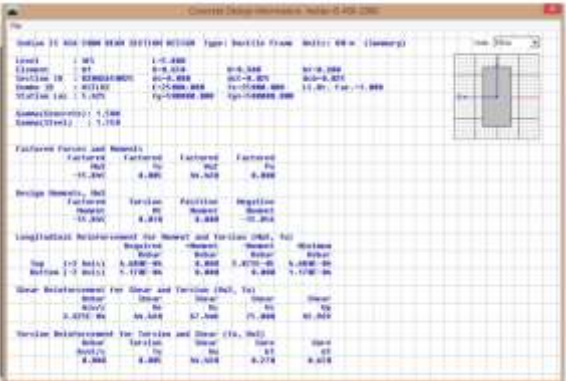


Detailing of beam for existing building (B600X750)

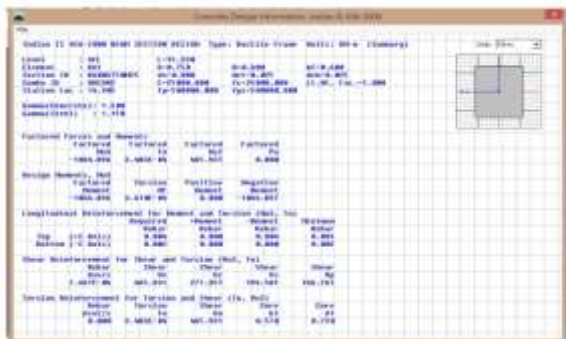


Detailing of beam for existing building (B750X750)

Design detail of column for retrofitting building



Detailing of beam for retrofitting building (B300X650)



Detailing of beam for retrofitting building (B600X750)



Detailing of beam for retrofitting building (B750X750)

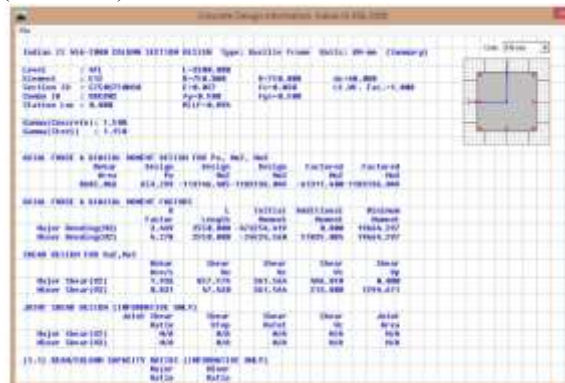
Design detail of column for existing building



Detailing of column for existing building (C300X700)

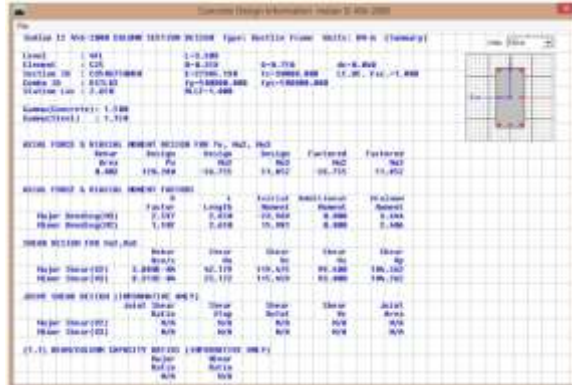


Detailing of column for existing building (C600X600)



Detailing of column for existing building (C750X750)

Design detail of column for retrofitting building



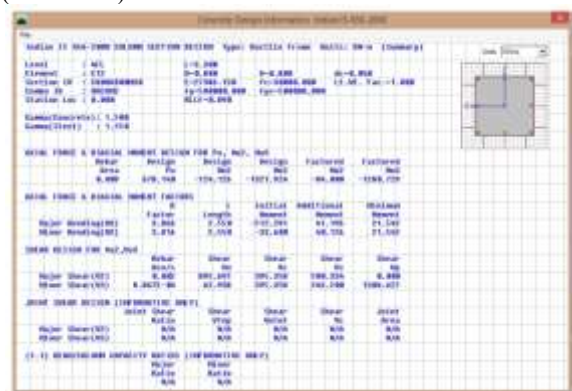
Level	Col	1 to 2	3 to 4	5 to 6	7 to 8
Column	C750	0-0.250	0-0.250	0-0.250	0-0.250
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750

Detailing of column for retrofitting building (C350X750)



Level	Col	1 to 2	3 to 4	5 to 6	7 to 8
Column	C350	0-0.250	0-0.250	0-0.250	0-0.250
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750

Detailing of column for retrofitting building (C650X650)



Level	Col	1 to 2	3 to 4	5 to 6	7 to 8
Column	C650	0-0.250	0-0.250	0-0.250	0-0.250
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750
Section ID	C080010000	0-0750, 750	0-0750, 750	0-0750, 750	0-0750, 750

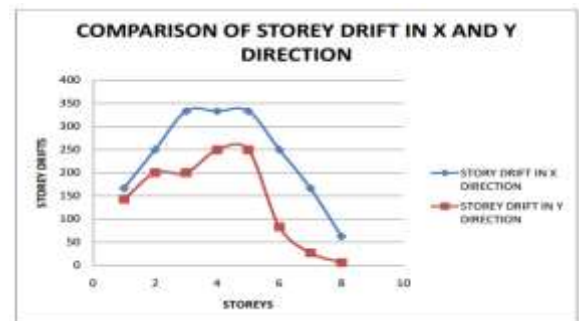
Detailing of column for retrofitting building (C800X800)

8.Results, conclusion and future research

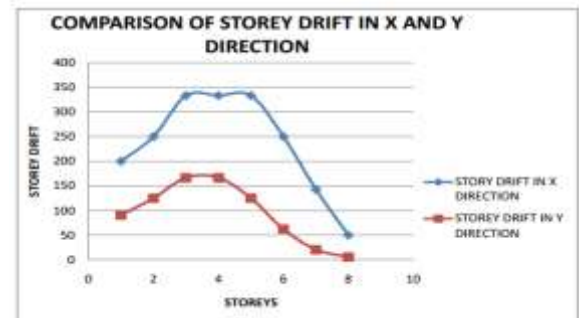
STORY DRIFTS (As per IS 1893-2002)

The storey drift in any storey due to the minimum specified design lateral force, with partial load factor of 1.0. shall not exceed 0.004 times the storey height, For the purposes of displacement requirements only (

see 7.11.1,7.11.2 and 7.11.3 only), it is permissible to use seismic force obtained from the computed fundamental period (T_n) of the building without the lower bound limit on design seismic force specified in 7.8.2

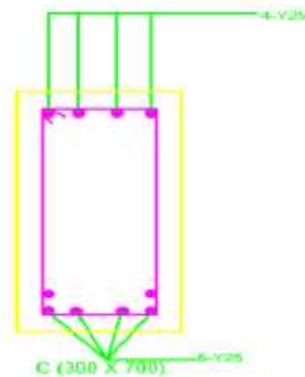


Comparison of storey drift for existing building

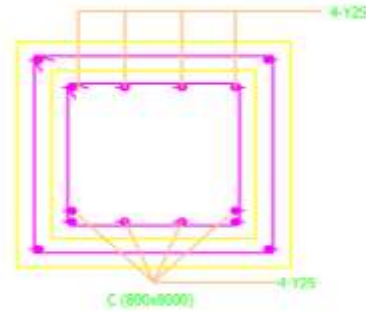


Comparison of storey drift for retrofitting building

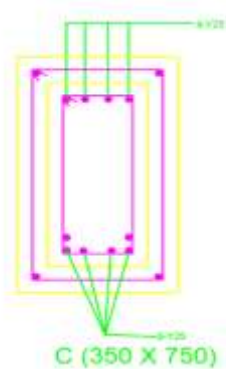
Detailing of the existing and retrofitting of column



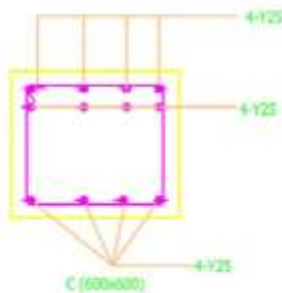
Cross section of existing column C (300X700)



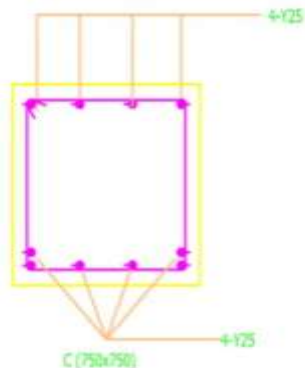
Cross section of retrofitting column C (800X800)



Cross section of retrofitting column C (350X750)



Cross section of existing column C (600X600)



Cross section of existing column C (750X750)

CONCLUSION:

After an extensive bibliographical study and the revision of several Indian design projects for the retrofit of structures, it can be concluded that there are some outlines that help, not only in a qualitative but in a quantitative manner, the design of a retrofit scheme of a structure by means of concrete jacketing. Although these guidelines can give a rational basis for a practical design, research still needs to address critical aspects in the behaviour of jacketed elements. The change in behaviour in jacketed elements whose shear span/death ratios are significantly reduced, due to their jacketing, needs to be clarified. In case of moderate seismic regions, reinforced concrete buildings are designed to resist dead and live loads only. This results in joint cores with reinforcement detail that cannot fulfill seismic design criteria, thus making such building frames vulnerable to joint shear failure when subjected to lateral loading or ground excitation. Even in seismic regions, old buildings designed according to the existing immature seismic design codes lack of sufficient hoops inside the joint cores. The joint cores are the most critical components in such frames, and the ultimate failure of such frames under lateral loading would be due to the inadequate shear capacity of the joint core. Hence, such lightly reinforced joints need to be strengthened by adding additional reinforcement by the method of retrofitting before exposing them to any form of lateral loading. Reinforced concrete (RC) jacketing is an effective method of retrofitting such connections. In this paper, the usefulness of reinforced concrete jacketing technique to strengthen lightly reinforced beam-column joints is investigated experimentally. A full-scale lightly reinforced concrete beam-column sub-assembly was strengthened by casting an reinforced concrete jacket outside of the column and at the joint and the improvement brought over by the retrofitting technique in the cyclic response of this specimen was

verified experimentally. The joint of the original specimen was not adequately reinforced to fulfill seismic design requirements, and it was the weakest component of the sub-assembly. When subjected to cyclic lateral loading, the joint panel will experience severe damage due to excessive shear deformation while the beam and column remained virtually undamaged. The original specimen was vulnerable to joint shear failure. On the other hand, the retrofitted specimen failed after the formation of a plastic hinge in the beam, and the joint was no longer the weakest component of the sub-assembly. Apart from the increase in the capacity and deformability of the joint, the shear deformation of the joint panel reduced significantly after retrofitting. It is concluded that the reinforced concrete jacketing method is effective in strengthening non-seismic reinforced concrete frames with inadequately reinforced joints.

Scope of Future Work

- Due to lack of design procedures in IS code, the design was implemented only for beams and columns. The project can be extended by suggestions on how to strengthen slabs. Also, schemes for shear strengthening of the failing members should be developed.
- The same 5-storey RC structure can be retrofitted using some different technique like base isolation, steel bracings, shear wall etc. and a comparative study can be done by modelling the structures in ETABS software.
- Find out the most efficient technique with respect to cost, aesthetics, durability and other such criteria.
- Lastly, fibre reinforced plastic wrapping is a relatively a new technique in which new genre of materials used in the realm of civil engineering and a lot of its properties can be determined. Future work needs to be done to determine its behaviour in specific conditions.

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