An Experimental Investigation On Replacement Of Cement With Sodium Polyacrylate And Lime Stone Waste

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ABSTRACT

Lime waste is an industrial waste which is obtained from the lime stone polishing industry in the form. This waste is easily carried away by the air and hence causing problems to human health and environmental pollution. The lime waste generated by the industry has accumulated over years, and it has been dumped unscrupulously resulting in environmental problem. Hence we are using lime waste as a cement replacement in different percentages and we have determined the compressive strength of concrete.

Concrete is a mixture of cement, aggregate, water or without suitable admixture. To attain desirable strength and serviceability curing is necessary. Concrete derives its strength by the hydration of cement particles, and thus process continuing for long time. The overall development of the country depends on infrastructure development for the uninterrupted progress of infrastructure availability of materials. In that materials water is the major material for the construction. Now a day's need of water is very high and availability is less. For that reason we use the polymers in the place of water. In this present work, it is aimed at developing a new building material from the lime scrap, an industrial waste as a replacement material of sodium polyacrylate and partial replacement of cement. Moreover, Sodium polyacrylate is very expensive wasting a very expensive material is not good engineering practice and hence Sodium polyacrylate is replaced by some amount of lime waste in certain percentages. By doing so, the objective of reduction of cost construction can be met and it will help to overcome the environmental problem associated with its disposal including the environmental problems of the region.

Keywords : Lime waste, Kurnool, Human health

INTRODUCTION

Lime waste is an industrial waste which is obtained from the lime stone polishing industry in the form. The waste is produced at Dhone and surrounding areas of Kurnool. Total waste produced by all industries may be approximately 2000 tones per week. This waste is easily carried away by the air and hence causing problems to human health and environmental pollution. The lime waste generated by the industry has accumulated over years, and it has been dumped unscrupulously resulting in environmental problem. Hence we are using lime waste as a cement replacement in different percentages and we have determined the compressive strength of concrete.

FIG: 1.1 Lime stone waste powder

EFFECTS OF DISPOSAL OF LIME WASTE

With the enormous increase in the quantity of waste needing disposal, acute shortage of dumping sites, sharp increase in the transportation and dumping costs affecting the environment, prevents sustainable development. The waste disposal problem is becoming serious. As it is a fine material, it will be easily carried away by the air and will cause nuisance causing health problems and environmental pollution. The major effect of air pollution are lung diseases, inhaling problems, the people who are living in and around are suffering from these problems.

OUR PROPOSAL:

In this present work, it is aimed at developing a new building material from the lime scrap, an industrial waste as a replacement material of sodium polyacrylate and partial replacement of cement. Moreover, Sodium polyacrylate is very expensive wasting a very expensive material is not good engineering practice and hence Sodium polyacrylate is replaced by some amount of lime waste in certain percentages. By doing so, the objective of reduction of cost construction can be met and it will help to overcome the environmental problem associated with its disposal including the environmental problems of the region.

1.4 OBJECTIVES:

Therefore, main objectives of the present work is

➤ To study the possibility to incorporate lime powder waste as a replacement of cement in addition to polymers.

➤ The other objectives of this work was to determine under what condition the lime powder in conjunctions with Sodium polyacrylate and plasticizer increases the strength of concrete when these are used as partial replacement materials of cement.

➤ To determine the degree of strength improvement in concrete with addition of lime stone powder

THEORETICAL ANAYSIS ON MATERIALS

An experimental study is conducted on lime powder produced from lime industry is mixed with cement concrete in partial replacement of cement. Normal strength grade concrete of M30 design mix with various percentages of lime powder and polymer replacing cement has been made use in the investigation. Experimental study is carried out to investigate the compressive strength of concrete with replacement of cement by polymer and lime powder.

CEMENT:

Ordinary Portland cement 53 grade Dalmia Brand conforming to B.I.S standards is used in the present investigations. The cement is tested for its various properties as per IS 4031-1988 and found to be conforming to the requirements as per IS 8122-1989.

LIME STONE POWDER:

A high quality lime stone powder generally permits a reduction in water content of a concrete mixture, without loss of workability. Lime stone powder obtained from India's cement Limited, DHONE was used for the study. Specific gravity of Lime stone powder used was 1.39.

FINE AGGREGATE:

The sand obtained from Hundri River near Kurnool is used as fine aggregate in this project investigation. The sand is free from clayey matter, silt and organic impurities etc. The sand is tested for specific gravity, in accordance with IS 2386-1963 and it is 2.65, where as its fine modulus is 2.31. The sieve analysis results are presented in table. The sand conforms to zone- III.

COARSE AGGREGATE:

Machine crushed angular Basalt metal from Tamma rajupalle near panyam used as coarse aggregate. The coarse aggregate is free from clayey matter, silt and organic impurities etc. The coarse aggregate is also tested for specific gravity and it is 2.72. Fineness modulus is 4.20. Aggregate of nominal size 20mm downgraded 50% retained on 12.5mm (passing) and 4.75mm (retained) is used in the experimental work, which is acceptable according to IS 383-1970.

WATER:

This locally available potable water, which is free from concentration of acid and organic substances .Is used for mixing the concrete

S.NO	PARAMETERS	RESULTS	PERMISSIBLE LIMITS AS PER IS 456-2000
1	Organic	46 mg/lit	200mg/lit
2	In organic	386 mg/lit	3000 mg/lit
3	Sulphates	40.32 mg/lit	400mg/lit
4	Chlorides	51.77 mg/lit	2000mg/lit Fire concrete not containing R.C.C For R.C.C 500mg/lit
5	Suspended matter	183 mg/lit	2000mg/lit
6	Ph	8.6	Not less than 6

NOTE: all the above parameters are within permissible limits, as mentioned in IS 456-2000.

PLASTICIZER
DEFINITION:

Plasticizers are combination of organic and inorganic substances which allow a reduction in water content for the given workability at the same water content, are termed as plasticizing admixtures.

PURPOSE OF PLASTICIZER:

The action of plasticizers is mainly to fluidity the mix and improves the workability of concrete, mortar or grout. Flowing concrete is also referred as self compacting concrete, collapsed slump concrete and flow concrete. This is the concrete having a slump equal to 200mm or more, a compaction factor of 0.98. Plasticizing admixtures are added to a concrete mixture to make plastic concrete extremely workable without additional water and corresponding loss of strength which makes it ideal for use in ready mixed concrete where workability is an important factor especially in places of congested reinforcement like beam column junction, heavy rafts and machine foundation, foundation of heavy structures.

As with conventional workability or plasticizing admixtures one can take advantage of the enhanced workability state to make reductions in water cement ratio of plasticized concrete while maintaining workability levels, namely 50 to 70mm slump concrete in this state is sometimes referred to as 'water produced' or 'high strength concrete'. The use of plasticizers in ready mixed concretes and construction reduces the possibility of detoriation of concrete for its appearance, density and strength. On the other hand, it makes the placing of concrete more economical by increasing productivity and reducing cost of labour in concrete handling and moving operations at the construction site.

Plasticizer used in the present project is Conplast SD 110

3.6.3 Description:

In the production of concrete blocks, CONPLAST will increase strength, improve rheological flow without increased slump, produce a denser block with better finished sides and corners and reduce the unit cost of the block

Conplast SD110 chloride-free admixture is specially formulated for optimum cost saving across a wide range of static and mobile (egg-laying) block paving and masonary production equipment. It is supplied as a brown solution which instantly disperses in water.

Conplast SD110 disperses the fine particles in the mix, improving cement dispersion and compaction to maximize the strength obtained from the cement used. The improved performance can then be used to allow reductions in cement content without loss of performance. Controlled air entrainment maintains yield and improves surface finish while providing improved resistance to frost attack.

Properties

Appearance   : Brown liquid
Specific gravity   : Typically 1.16
at 20 C
Chloride content   : Nil to BS 5075
Alkali content   : Typically less
than 5.0 g. Na2O
Conplast SD110 is suitable for use with all types of Ordinary Portland cements and cement replacement materials such as Lime powder and polymers.

Advantages:

- ❖ Increase in strengths, density and yield of semi-dry concrete units.
- ❖ It can be maintained at lower cement contents.
- ❖ Allows higher early and later age strengths to be obtained without additional cement.
- ❖ Improves strength, resulting in fewer losses due to breakage.

- ❖ Air entrainment improves resistance to frost attack.
- ❖ Improved mix cohesion allows economical use of a wide range of aggregates.
Typical dosage
- ❖ The optimum dosage of Conplast SD110 to meet specific requirements should always be determined by trails using the materials and conditions that will be experienced in use.
- ❖ This allows the optimization of admixture dosage, mix design and equipment settings and provides a complete assessment of the final product. A starting point for such trials is to use a dosage within the normal typical range of 0.20 to 0.50 liters/ 100kg of cementitious material

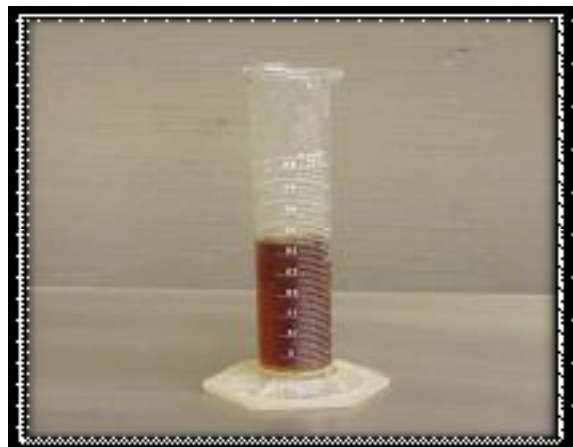**Fig 3.6.1 CONPLAST SD 110**

Mix design for M30 grade of concrete

1) **Design stipulation:- (as per IS 10262-2009 and IS 456-2000)**
   a) Grade designation - M30
   b) Type of cement   - OPC 53grade
   c) Maximum nominal size of aggregate
      - 20mm
   d) Maximum water-cement ratio
      - 0.45(IS 456)
   e) Workability   - 100mm(Slump)
   f) Exposure condition
      - Severe(for R.C.C)

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g) Degree of super vision   - Good
h) Type of aggregate
            - Crushed angle aggregate
i) Chemical admixture   - Plasticizer
j) Minimum cement content(for severe) - 320 kg/m3 (IS 456:2000)

2) **Test data for materials:**
   a) Cement specific gravity   - 3.1
   b) Compressive strength of cement at 7 days   - 26.8
   c) Specific gravity of coarse aggregate   - 2.72
   d) Specific gravity of fine aggregate   - 2.65
   e) Water absorption coarse aggregate   - 0.5%
   f) Water absorption fine aggregate   - 1%
   g) Free(surface) Moisture of coarse aggregate   - Nil
h) Free(surface) Moisture of fine aggregate   - Nil

i) **Target mean strength for mix proportioning(M30):**
   Ft = f ck + 1.65s
   Where
   Ft = Target average compressive mean strength at 28 days
   f ck   = Characteristic compressive mean strength at 28 days
   s   = Standard deviation= 5n/mm2 for M30 grade of concrete(from table 1 of IS 10262-2009)
   Target mean strength ft = 30+1.65(5) = 38.25 N/mm2

j) **Selection of water-cement ratio:-**
   From table 5 of IS 456-2000
   Maximum water-cement ratio = 0.45
   Based on experience adopt water-cement ratio   = 0.39

5) **Strength of water content:**
   From table 2 of IS 10262-2009
   Maximum water content = 186 lit
   (This is for 25 to 50 mm slump range)
   Estimated water content for 100 mm slump = 186 + 6% of w. c

(Clause 4.2 of IS 10262-2009)
= 186 + 6% (186) = 197 lit
     As plasticizer is used the w. c can be reduced up to 20% and above. Based on trails with plasticizer water content reduced of 29% has been achieved hence the arrived w. c 197-29% (197) = 139.87 lit = 140 lit

7) **Calculation of cement content:**
   Water-cement ratio =0.39
   Cement content = 140/0.39=358.97
          =360 kg/m3
   From table 5of IS 456-2000 minimum cement content for severe exposure condition =320 kg/m3
   360 kg/m3 > 320 kg/m3
   Hence ok

8) **Proportion of volume of coarse aggregate and fine aggregate:**
   From table 3 of IS 10262-2009
   Volume of coarse aggregate corresponding to 20mm size aggregate and fine aggregate (zone-III) for Water-cement ratio of 0.5=0.64
   In present case Water-cement ratio is 0.39
   Volume of coarse aggregate is required to be increased to decrease the fine aggregate content. As the water-cement ratio is lowered by 0.11 the proportion of volume of coarse aggregate is increased by 0.022.
   The corrected proportion of Volume of coarse aggregate for Water-cement ratio 0.39= 0.64+0.022   = 0.662
   The Volume of coarse aggregate   = 0.66
   Volume of fine aggregate= 1-0.66 =0.34

9) **Mix Calculations:**
The mix calculations are per unit volume of concrete shall be follows
   a) Volume of concrete   = 1m3
   b) Volume of cement = mass of cement/sp. gravity of cement*1/1000
      =360/3.1*1/1000 = 0.115 m3
   c) Volume of water   = mass of water/sp. gravity of water*1/1000

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=140/1*1/1000 = 0.140m3

d)Volume of chemical admixture (plasticizer)@ 2% of mass of cementious material = mass of admixture/sp. gravity of admixture *1/1000

= 7.2/1.01*1/1000 = 0.0071m3

e) Volume of all aggregates=[a-(b+c+d)]

=[1-(0.116+0.14+0.0070)] = 0.737

Mass of coarse aggregate = (Volume of all aggregate)*(Volume of coarse aggregate)*(sp. gravity of coarse aggregate)*1000

= 0.738*0.662*2.71*1000

= 1319.98kg

g) Mass of fine aggregate =
0.738*0.34*2.65*1000

= 664.93kg = 665kg

Final Mix

0.39 : 1 : 1.84 : 3.68
Water Cement Fine Agg Coarse Agg

EXPERIMENTAL TESTS ON MATERIALS

TEST ON CEMENT

4.1 Fineness of cement:-

AIM:

To determine the fineness of the cement by dry-sieving method

APPARATUS:

I.S. Sieve No.9 (90 microns),
Weighing balance capacity 5kg

OBSERVATIONS:

Brand of cement: OPC 53 grade

Trail No.	1	2	3
Weight of cement(g)	500	500	500
Weight of residue in I.S. Sieve No.9	12	15	15
Quantity of cement retained (%)	2.4	3	3

Table 4.1.1 Fineness of cement

RESULT: Fineness of the cement = 2.8%

4.2. STANDARD CONSISTENCY OF CEMENT PASTE

AIM: To determine the normal consistency and initial and final setting time of cement paste

APPARATUS: Vicat's apparatus with plunger and needles, stopwatch etc

% age of water	Initial reading	Final reading	Depth Penetrated[mm]
110	0	19	19
120	0	15	15
130	0	10	10
135	0	8	8
138	0	5	5

Table 4.2.1 Standard consistency

Initial reading is the indicator reading when the lower and of the plunger touches the bottom non-porous paste.

RESULT: Standard consistency of cement :(138/400)*100 = 34.5

4.3. SOUNDNESS OF CEMENT (BY LECHATLIER METHOD):-

AIM:

To determine the soundness of cement

APPARATUS:

Lechatlier apparatus is used for the determination of soundness of cement. It consists of a small split cylinder of spring brass of 0.5mm thickness, forming a mould 30mm internal diameter and 30mm high. On either side of split are attached two indicators with pointed ends the distance from these ends to centre of the cylinder being 165mm.The mould should be kept in good condition with the jaws more than 0.5mm apart.

OBSERVATIONS

Type of cement: OPC 53 grade
Normal consistency= 34.5%
Water required for soundness test = 0.78*p=27ml
Initial distance =10mm
Final distance = 11mm

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Expansion of cement = 1mm
RESULT:
Soundness of the cement: 1mm

4.4. SPECIFIC GRAVITY OF CEMENT:

AIM:
 To determine the specific gravity of cement of the cement paste

APPARATUS:
 Le-chatlier flask, weighing balance, water bath, kerosene, thermometer

OBSERVATIONS:
Brand of cement: DALMIA OPC 53 grade
Liquid used: kerosene
Density of liquid at room temperature: 0.8
 Weight of cement taken: 64g

S.NO	Initial reading	Final reading	Volume of cement(v) in c.c.	Specific gravity G=W/V
1	0	21	21	3.04
2	0	21	21	3.04

Table 4.4.1 Specific gravity of cement

RESULT:
   Specific gravity of the given cement : 3.04@ room temp.

4.5 COMPRESSIVE STRENGTH OF CEMENT:-

AIM:
This method of tests covers the procedure for determining the compressive strength tests on mortar cubes compacted by means of a standard vibration machine

STANDARD SAND:
The standard sand to be used in the test shall conform to IS: 650-1966(specification for standard sand for testing of cement)

CALCULATIONS:
Weight of cement taken = 200g
Weight of sand taken =600g
Water added = (p/4+3)%of(cement, sand)=34.5
Average compressive strength of cement under loading for 7 days=26.8

Average compressive strength of cement under loading for 28 days=34.2

RESULT:
Characteristic compressive strength of given cement sample is 34.2

TESTS ON AGGREGATES:-
4.6. GRAIN SIZE DISTRIBUTION OF FINE AGGREGATE:

AIM:
To study the particle size distribution of the given fine aggregate by doing sieve analysis and to draw the grading Curve.

APPARATUS:
Set of sieves consists 4.75mm,2.36mm,1.18mm,600micron,300mic,150mic and pan, lid, weighing balance with   accuracy up to 0.1grms , weights and sieve shaker.

Sieve size	% passing for grading			
	I	II	III	IV
10mm	100	100	100	100
4.75mm	90-100	90-100	90-100	95-100
2.36mm	60-95	75-100	85-100	95-100
1.18mm	30-70	55-90	75-100	90-100
600mic	15-34	35-39	60-79	80-100
300mic	5-2	8-30	12-40	15-50
150mic	0-10*	0-10*	0-10*	0-15*

PREPARATION OF SAMPLE:
    The sample should be taken by quartering for which the sample is thoroughly mixed and spread over a clean surface. It is then cut into four equal parts by trowel care being taken to include finer and due. Two opposite quarters are taken and mixed makes the sample.

    If any further reduction of quantity is required the process may be repeated. Weight retained on each sieve shall not exceed the limits specifies by the IS code.

S. No	Sieve size	Wt. retained in g	%age wt. retained	Cumulative %age wt .retained(F)	%age passing (100-F)
1	4.75	-	-	-	100
2	2.36	25	2.5	2.5	97.5
3	1.18	130	13	15.5	84.5
4	600	210	21	36.5	63.5
5	300	500	50	86.5	13.5
6	150	120	12	98.5	1.5

Table: 4.6.1 Sieve analysis for fine aggregate

CALCULATIONS:

Fineness modulus=240.5/100=2.4

4.7 SPECIFIC GRAVITY OF COARSE AGGREGATE AND FINE AGGREGATE:

AIM:

To determine the specific gravity of coarse aggregate and fine aggregate

APPARATUS:

Weighing balance(capacity not less than 3kg), pycnometer ,drying oven, shallow metal tray(area 32500mm2) air tight container and dry absorbent clothes each not less than 77*45cms

OBSERVATIONS:

For 20mm size:

Weight of pycnometer (w1) = 420g
Weight of pycnometer +coarse aggregate (w2)   = 925g
Weight of pycnometer+ coarse aggregate + water (w3)   = 1760g
Weight of pycnometer + water (w4)
                                = 1440g

For 10mm size:

Weight of empty pycnometer (w1) = 420g
Weight of pycnometer + coarse aggregate (w2)   = 875g
Weight of pycnometer + coarse aggregate + water (w3)   = 1680g
Weight of pycnometer + water (w4)
                                = 1395g

For fine aggregate:

Weight of pycnometer (w1)= 420g
Weight of pycnometer + fine aggregate (w2)   = 1000g
Weight of pycnometer + fine aggregate + water (w3)   = 1760g
Weight of pycnometer + water (w4)
                                = 1445g

CALCULATIONS:

Specific gravity = W2-W1/ (W2-W1)-(W3-W4)
= (925-420)/ (925-420)-(1760-1440)
            = 2.72

Specific gravity = W2-W1/ (W2-W1)-(W3-W4)
 = (875-420)/ (875-420)-(1680-1395)
 = 2.72

Specific gravity = W2-W1/ (W2-W1)-(W3-W4)
 = (1000-495)/ (1000-495)-(1760-1445)
 = 2.65

RESULT:

Specific gravity of the coarse aggregate of 20mm, 10mm size and fine aggregate is 2.72, 2.72, 2.65 respectively.

TESTS ON FRESH CONCRETE

4.8 Slump cone test

AIM:

To determine the slump of the given material

APPARATUS:

Mould, tamping rod, trowel

PROCEDURE:

❖ Fill the concrete into frustum of a steel cone in three layers.

❖ Hand tap concrete with 25 strokes in each layer

❖ Lift come up without lateral or torsion motion

❖ Immediately measure the difference between the height of the mould and the highest point of the specimen being test.
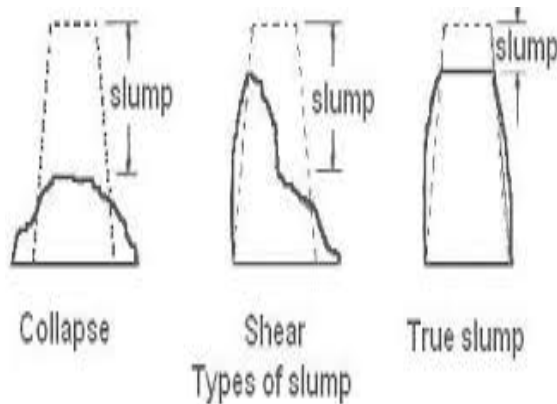
❖ Slump test that shear or collapse must be repeated.

Fig 4.8.1 Slump cone test

TEST RESULT:

Slump in terms of millimeters to the nearest 5mm = 165mm

Shape of the slump: true

Referring to the selection of data, we have a slump value within the range (60-180mm)

COMPACTION FACTOR TEST:

AIM:

To determine the compaction factor by using compaction factor test

APPARATUS:

Two hoppers, one cylinder

PROCEDURE:

- ❖ Concrete is placed in an upper hopper
- ❖ Dropped into lower hopper to bring it to a standard state and then allowed to fall into a standard cylinder
- ❖ The cylinder and concrete weighed(partially compacted weight)
- ❖ Concrete is placed in the cylinder in 6 layers and compacted each layer with 30 strokes of the tamping rod.
- ❖ Weigh the concrete and cylinder(fully compacted weight)

Fig 4.9.1 compaction factor apparatus

Partially compacted concrete:

Cylinder weight=5.833kg

Cylinder+ concrete weight=17545.2

Weight of concrete=17.545-5.833 = 11.66kg

Fully compacted concrete:

Cylinder compacted concrete weight=17750g

Weight of concrete=17.750-5.833=11.867kg

Compaction factor=Weight of partially compacted concrete/Weight of fully compacted concrete

=11.61/11.867=0.98

RESULT:

Compaction factor = 0.98

TESTS ON HARDENED CONCRETE

Compressive strength test:

- ❖ Concrete is primarily meant to withstand compressive stresses.
- ❖ Cubes, cylinders and prisms are the three types of compression test specimens used to determine the compressive strength.
- ❖ Cubes of size 150×150×150mm are used in the present work.

The specimens are casted as follows:
- ❖ The mould is applied with oil for lubrication.
- ❖ Concrete is laid in the mould in a layer up to some height and compacted with a tamping rod.

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❖ In this way, the concrete is laid in three layers and the procedure is repeated.
❖ The next step is vibration on a vibrating machine.
❖ The above procedure is the same for all the mixes with different percentage of admixture replacement.
❖ The cubes are cured for 28 days.
❖ After 28 days of curing, the cubes are tested in a compression testing machine (CTM).
❖ The values obtained are tabulated.
❖ Compressive strength of a cube =load/area.

Samples	Load	Compressive strength(N/mm2)
1	970	43.1
2	1000	44.5
3	1030	45.6

Table 4.10.1 Compressive strength

RESULT: The average of 3 samples is taken as the compressive strength of cube= 44.4 N/mm^2

4.11 Flexural Strength:

The determination of flexural strength is essential to estimate the load at which the concrete members may crack. The flexural strength at failure is the modulus of rupture.The modulus of rupture is determined by testing standard test specimens of size $100 \times 100 \times 500$ mm over a span of 400 mm under Two point loading.

Bending Tensile stress or Flexural Strength = PI/bd^2 when a> or = 40/3 cm

3Pa/bd^2 when $40/3 \geq a \geq 11$cm

RESULTS AND DISCUSSION

Replacement of cement by Sodium polyacrylate

(a) Influence of sodium poly acrylate on compression strength
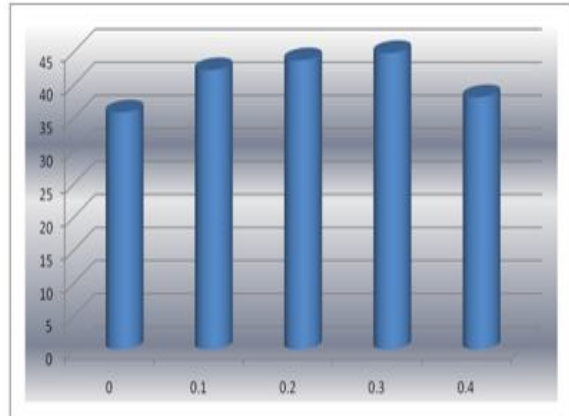Compressive strength Vs % of cement replacement by Sodium polyacrylate

Fig 5.1.1 Compressive strength Vs %of cement replacement by Sodium polyacrylate

S.NO	%Replacement of cement with sodium polyacrylate	Compressive strength in (N/mm2)	% increase in strength
1	0	35.8	–
2	0.1	42.2	17.87
3	0.2	43.7	22.06
4	0.3	44.8	25.13
5	0.4	38.1	6.42
6	0.5	36.2	1.11
7	0.6	34.2	–

Table 5.1.1 Compressive strength Vs %of cement replacement by Sodium polyacrylate

Mix design	Nominal mix	mix-1	mix-2	mix-3	mix-3	mix-4	mix-5
%replacement of sodium polyacrylate	0	0.1	0.2	0.3	0.4	0.5	0.6
w/c ratio	0.39	0.39	0.39	0.39	0.39	0.39	0.39
Cement content(kg)	1.9	1.89	1.89	1.89	1.88	1.88	1.88
Fine aggregate(kg)	2.16	2.16	2.16	2.16	2.16	2.16	2.16
Coarse aggregate	4.30	4.30	4.30	4.30	4.30	4.30	4.30
Water(lit)	0.712	0.712	0.712	0.712	0.712	0.712	0.712
Compressive strength 28days(N/mm2)	35.8	42.2	43.7	44.8	38.1	36.2	34.2

In the present investigation sodium poly acrylate has been used as replacement of cement up to a maximum of 20%. The compressive strength for different percentage of sodium poly acrylate and

percentage increase or decrease in strengths with respect to M30 grade concrete listed in the table. By taking normal M30 grade as referring percentage, percentage of increase or decrease in compressive strength other percentage is calculated. Considering the normal M30 with zero percentage admixtures the compressive strength is 35.8 N/mm2. When 0.1% replacement is used, the compressive strength is 42.2 N/mm2. Considering 0.2% replacement, the compressive strength is 43.7 N/mm2. And there is an increase in the strength 22.06 N/mm2. With 0.3% replacement, the compressive strength is 44.8 Nimm2 and there is a increase in strength 25.13 N/mm2. With 0.4% replacement, the compressive strength is 38.1 and there is a little increase in the strength. However, 0.4% can be taken as optimum dosage which can be mixed in cement concrete for giving optimum possible compressive strength at any stage.

5.2 Replacement of cement by Lime stone powder, Clinkers and Super plasticizer

(b) Influence of Lime stone powder, Clinker and Super plasticizer on compression strength
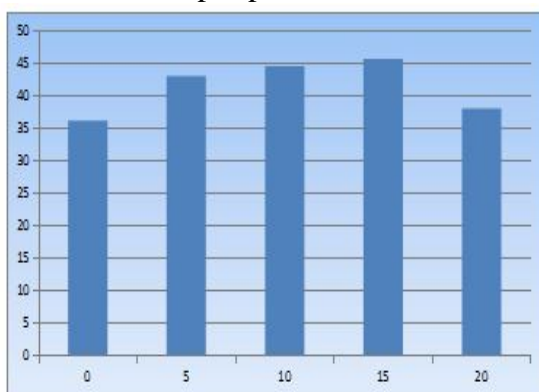Compressive strength Vs %of cement replacement by Lime stone powder, Clinkers and Super plasticizer

Fig 5.2.1 Compressive strength Vs %of cement replacement by Lime stone powder, Clinkers and Super plasticizer

S.NO	% Replacement of cement with Lime stone powder	Compressive strength in (N/mm2)	% increase in strength
1	0	36.2	_
2	5	43.1	19.06
3	10	44.5	22.92
4	15	45.6	25.96
5	20	38.1	5.24

Table 5.2.1 compressive strength of Lime stone powder, Clinkers and Super plasticizer

Mix design	Nominal mix	mix-1	mix-2	mix-3	mix-4
%replacement of Lime stone powder	0	5	10	15	20
w/c ratio	0.39	0.39	0.39	0.39	0.39
Cement content(kg)	1.35	1.28	1.21	1.14	1.08
Fine aggregate(kg)	2.5	2.5	2.5	2.5	2.5
Coarse aggregate	4.94	4.69	4.44	4.19	3.95
Water(lit)	0.525	0.525	0.525	0.525	0.525
Plasticizer (lit)	0.025	0.025	0.025	0.025	0.025
Limestone powder(g)	33.75	67.5	135	202.5	270
Clinkers (g)	123.5	247	494	741	988
Compressive strength 28days (N/mm2)	36.2	43.1	44.5	45.6	38.1

In the present investigation Lime stone powder has been used as replacement of cement up to a maximum of 20%. The compressive strength for different percentage of sodium poly acrylate and percentage increase or decrease in strength with respect to M30 grade concrete listed in the table. By taking normal M30 grade as referring percentage, percentage of increase or decrease in compressive strength other percentage is calculated.

Considering the normal M30 grade with zero percentage admixtures the compressive strength is 36.2N/mm2. When 5% replacement is used, the compressive strength is 43.1 N/mm2 and increase in strength is 19.06 N/mm2.

Considering 10% replacement, the compressive strength is 44.5N/mm2. And there is an increase in the strength is 22.92 N/mm2. With 15% replacement, the compressive strength is 45.6 N/mm2 and

there is a increase in strength 25.96 N/mm2 .With 20% replacement, the compressive strength is 38.1 and there is a little increase in the strength. However, 15% can be taken as optimum dosage which can be mixed in cement concrete for giving optimum possible compressive strength at any stage.

05.3 Split tensile Strength

(c) Influence of both Sodium polyacrylate and lime stone powder on split tensile strength
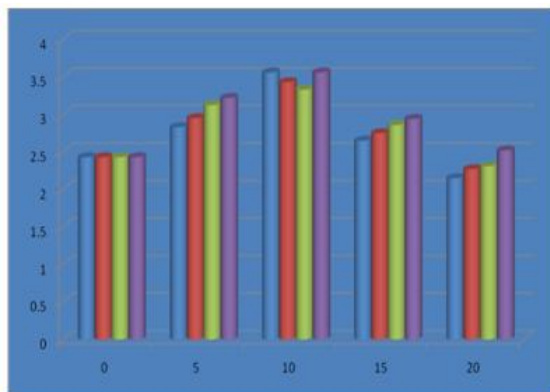

Fig 5.3.1 Split tensile Strength Vs % of combination of sodium polyacrylate and lime stone powder

S.NO	%age both sodium polyacrylate and lime stone powder	Sodium polyacrylate	Lime stone powder	Replacement of (S+L)	Addition of (S+L)
1	0	2.43	2.43	2.43	2.43
2	5	2.83	2.95	3.125	3.22
3	10	3.56	3.43	3.33	3.56
4	15	2.65	2.75	2.86	2.94
5	20	2.15	2.27	2.31	2.52

Table 5.3.1 Split tensile strength of both sodium polyacrylate and lime stone powder

Mix design	Nominal mix	mix-1	mix-2	mix-3	mix-4
%replacement or addition	0	3+2=5	6+4=10	9+6=15	12+8=20
w/c ratio	0.39	0.39	0.39	0.39	0.39
Cement content (kg)	2.28	2.16	2.05	1.93	1.82
Fine aggregate(kg)	4.19	4.19	4.19	4.19	4.19
Coarse aggregate	8.31	8.31	8.31	8.31	8.31
Water(lit)	0.882	0.882	0.882	0.882	0.882
Plasticizer(lit)	0.045	0.045	0.045	0.045	0.045
Split tensile strength (N/mm^2) Sodium polyacrylate	2.43	2.83	3.56	2.65	2.15
Split tensile strength (N/mm^2) lime stonepowder	2.43	2.95	3.43	2.75	2.27
Split tensile strength of (Sodium polyacrylate and lime stone powder) replacement(N/mm^2)	2.43	3.12	3.33	2.86	2.31
Split tensile strength of (Sodium polyacrylate and lime stone powder) (N/mm^2) addition	2.43	3.22	3.56	2.94	2.52

5.4 Flexural Strength

(d) Influence of both sodium polyacrylate and lime stone powder on flexural tensile strength

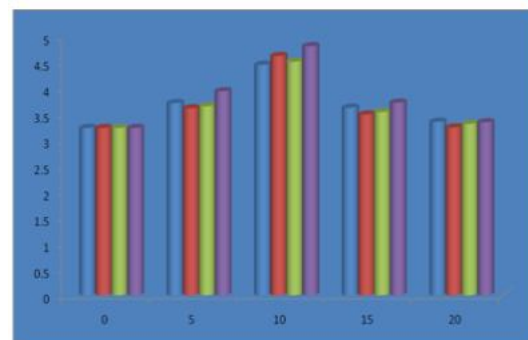Flexural tensile strength Vs % of both sodium polyacrylate and granite powder

Fig 5.4.1 flexural strength Vs% of combination of sodium polyacrylate and lime stone powder

S.NO	%age both sodium polyacrylate and lime stone powder	Sodium polyacrylate	Lime stone powder	Replacement of (S+L)	Addition of (S+L)
1	0	3.23	3.23	3.23	3.23
2	5	3.71	3.61	3.65	3.94
3	10	4.45	4.62	4.51	4.81
4	15	3.62	3.49	3.54	3.72
5	20	3.35	3.24	3.31	3.34

Table 5.4.1 flexural strength of both sodium polyacrylate and lime stone powder

Mix design	Nominal mix	mix-1	mix-2	mix-3	mix-4
%replacement or addition	0	3+2=5	6+4=10	9+6=15	12+8=20
w/c ratio	0.39	0.39	0.39	0.39	0.39
Cement content (kg)	2.16	2.05	2.03	1.92	1.80
Fine aggregate(kg)	3.99	3.99	3.99	3.99	3.99
Coarse aggregate	7.91	7.91	7.91	7.91	7.91
Water(lit)	0.84	0.84	0.84	0.84	0.84
Plasticizer(lit)	0.043	0.043	0.043	0.043	0.043
Flexural Strength (N/mm^2) Sodium polyacrylate	3.23	3.71	4.45	3.62	3.35
Flexural Strength (N/mm^2) lime stonepowder	3.23	3.61	4.45	3.62	3.35
Flexural Strength of (Sodium polyacrylate and lime stone powder) replacement(N/mm^2)	3.23	3.65	4.51	3.54	3.31
Flexural Strength of (Sodium polyacrylate and lime stone powder) (N/mm^2) addition	3.23	3.94	4.81	3.72	3.34

CONCLUSIONS

Based on limited experimental investigations conducted the following Conclusions are made

- From observations it can be concluded that Lime stone powder as a substitute giving better strength than Sodium polyacrylate or a Combination of Sodium polyacrylate and Lime stone powder.
- With partial replacement of 0-15% of Lime stone powder by cement the compressive strength increases.
- It may be economical by using Lime stone powder instead of Sodium poly acrylate, Since Sodium Polyacrylate is a costliest material.
- It can be concluded that method of curing has considerable effect on the mechanical properties including compressive, flexural and shear strength of SCC.
- If we use Lime waste in concrete cost of the project can be reduced to some extent.

- By using the Lime stone powder waste as filler in concrete or replacement in cement will reduce environmental pollution.

FUTURE SCOPE OF WORK

The effect of different curing methods on mechanical properties of self compacted concrete is studied i.e. effect on Compressive, Flexural & Shear strength was studied. The following recommendations are suggested for the future research:

- Effect of curing on other properties like Modulus of Elasticity, Bond etc.
- The curing mechanism may be studied by study of microstructure in more details.
- Effect of change of molecular weight of PEGs on self curing capacity.
- Other methods of self curing may be studied such as Lightweight Aggregate (natural and synthetic), LW Sand, Super-absorbent Polymers etc.
- The durability performance of SCSCCC such as resistances to corrosion, alkali-aggregate reaction, sulfate attack, and freezing and thawing should be investigated.
- Effect of natural climate factors such as sunlight, ambient temperature and humidity during self curing on the properties may be studied**.**

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